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# ▶ To cite this version:

Martin Auroy, Stéphane Poyet, Patrick Le Bescop, Jean-Michel Torrenti, Thibault Charpentier, et al.. Impact of carbonation on unsaturated water transport properties of cement-based materials. Cement and Concrete Research, 2015, 74, pp.44-58. 10.1016/j.cemconres.2015.04.002 . cea-01272793

# HAL Id: cea-01272793 https://cea.hal.science/cea-01272793v1

Submitted on 11 Feb 2016  $\,$ 

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# Impact of carbonation on unsaturated water

# <sup>2</sup> transport properties of cement-based materials

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# 16 Abstract

- 17 In unsaturated conditions, the durability of concrete structures is strongly dependent on the evolution
- 18 of the amount of free water within concrete porosity. Reliable durability assessment of concrete
- 19 structures in relation to their environment thus requires accurate unsaturated water transport
- 20 description as well as reliable input data. The effect of carbonation on water transport remains poorly
- 21 studied and data are lacking. It was then the purpose of this article to acquire all the data needed to
- 22 describe unsaturated water transport in carbonated cementitious materials (porosity, water retention
- and unsaturated permeability). Four hardened pastes made with four different binders were
- 24 carbonated at 3% CO<sub>2</sub> to ensure representativeness with natural carbonation. Beyond the modification

25 of the water retention curve and porosity clogging, significant microcracking due to carbonation

shrinkage was observed. The consequence on permeability highlighted a competition between

27 porosity clogging and microcracking that was dependent on the initial mineralogical composition.

28 Keywords: waste management (E); cement paste (D); drying (A); permeability (C); microstructure (B)

### 29 **1. Introduction**

30 In unsaturated conditions, the durability of concrete structures is strongly dependent on the evolution 31 of the amount of free water within concrete porosity (the term "free water" means the water that can be 32 evaporated: it includes capillary and adsorbed water). This dependence is well illustrated by the 33 results of Tuutti [1] that show the strong evolution of the corrosion current (over several orders of 34 magnitude) of steel embedded in a carbonated mortar as a function of the external relative humidity 35 (RH). In a more general way water significantly influences concrete performances and durability 36 (shrinkage, creep, cracking, transport properties). Reliable durability assessment of concrete 37 structures in relation to their environment thus requires accurate unsaturated water transport 38 description. Many studies were published on this specific subject and a lot of data were acquired in the 39 laboratory using different cement-based materials taking care to avoid interaction with carbon dioxide 40 (CO<sub>2</sub>). These results are of course very important but they are not fully relevant for durability appraisal 41 because they disregard the fact that concrete structures are being carbonated when exchanging water 42 with the environment.

Carbonation refers to the reaction between the calcium contained in concrete pore solution and
gaseous CO<sub>2</sub>. It leads to hydrates dissolution (mainly portlandite and C-S-H) and precipitation of
calcium carbonate (CaCO<sub>3</sub>). Beyond the fall of the pore solution pH that triggers rebar depassivation
and corrosion, the precipitation of CaCO<sub>3</sub> generates porosity clogging: the reduction of which depends
on the cement type and water to cement ratio (w/c) [2-8]. The specific surface area is then significantly
decreased (around 50%) despite conflicting observations [9, 10].

The pore size distribution is also altered. Litvan and Meyer [11] studied carbonated concrete samples taken from a 20-year field exposure test (two concretes made with CEM I and CEM III). They found that carbonation led to significant coarsening of the pore structure of the CEM III concrete whereas the finer pores were affected for the CEM I one. Using pastes with different w/c (from 0.4 to 0.7) Ngala 53 and Page [5] found that carbonation (using 5% of CO<sub>2</sub>) increased the proportion of capillary pores 54 (pores larger than 30 nm) whatever the binder and water to binder ratio. More recently, Thiéry et al. [7] 55 carbonated CEM I pastes with different w/c (using 50% of CO<sub>2</sub>) and found that carbonation was 56 capable of producing large capillary pores (larger than 100 nm) for w/c greater than 0.45. This could 57 be just a consequence of the high CO<sub>2</sub> content used in this study because the authors noted that this 58 phenomenon was reproduced using 10% of CO<sub>2</sub>. Morandeau [12, 13] conducted similar experiments 59 using CEM I pastes with or without fly ash (using 10% of CO<sub>2</sub>). The pore volume of the CEM I pastes 60 was almost uniformly reduced over the whole pore size distribution whereas a significant coarsening 61 was obtained for pastes containing fly ash.

62 The water retention curves (*i.e.* adsorption and desorption isotherms) are also transformed. Due to 63 porosity clogging, the water retained at equilibrium is significantly reduced [2, 4, 14-16]. In addition, using CEM I pastes of different w/c Houst [14], Houst and Wittmann [4] (≈90% CO<sub>2</sub>) and Thiéry et al. 64 [7] (50% CO<sub>2</sub>) showed that despite this reduction in water content (in percent by dry mass) the curves 65 66 remained unchanged when they were expressed in saturation (Figure 1). This highlights the change in density induced by carbonation due to CO<sub>2</sub> fixation (1.60 and 2.03 g/cm<sup>3</sup> for non- and carbonated 67 pastes respectively as estimated using mercury intrusion porosimetry [14]). This also means that all 68 69 the pores were impacted in the same manner by CaCO<sub>3</sub> precipitation. This is of course not consistent 70 with the coarsening of the pore structure discussed above. This is also not consistent with the results 71 of Hyvert [16] who obtained significant alteration of the water retention curve of CEM I and CEM III/A 72 mortars (w/c = 0.5) after carbonation (50%  $CO_2$ ) (Figure 2).



Figure 1: Effect of carbonation on the desorption isotherm of a CEM I paste (w/c=0.4), redrawn
 after [14]. Key: ● non-carbonated paste, ▲ carbonated paste.





77 The effect of carbonation on permeability is also controversial. Martin [17] used  $CO_2$  in gas 78 permeability tests and observed a reduction in permeability as a consequence of carbonation during 79 the test. Using CEM I grouts, Dewaele et al. [9] obtained permeability decrease of several orders of 80 magnitude after carbonation (by injection under pressure of CO<sub>2</sub>-rich water). Claisse et al. [6] 81 carbonated CEM I concretes (5% CO<sub>2</sub>) and measured significant increase in the so-called 82 impermeability index [18] indicating a fall in permeability. In the same way Song and Kwon [19] 83 carbonated two CEM I concretes and found that the permeability coefficient of the carbonated 84 concretes was three times less than that of the non-carbonated ones. These results are however not 85 consistent with the observations of Borges et al. [20] who measured oxygen permeability on CEM I 86 paste with or without slag (75% and 90%). The CEM I paste permeability values remained stable after 87 carbonation (5% CO<sub>2</sub>) whereas the pastes incorporating slag showed significant increase in 88 permeability. Moreover Thiéry et al. [7] and Wang et al. [21] showed that for a high w/c CEM I concrete 89 (0.84) the permeability (evaluated using inverse analysis) was increased after carbonation (by one 90 order of magnitude).

In summary, there are not enough studies dealing with the effect of carbonation on water transport
properties and the results are piecemeal and often conflicting. This might be due to differences in CO<sub>2</sub>
content, cement type or even mix composition. It is then the purpose of this article to study the effect
of carbonation and to acquire all the data needed to describe unsaturated water transport in
carbonated cement pastes; that is to say: porosity, water retention curve and unsaturated permeability
(see section 2).

# 97 2. Theoretical framework

Water transport within concretes involves three mechanisms: (*i*) permeation of the liquid phase; (*ii*) permeation of the gaseous phase (mix of vapour and dry air) and (*iii*) diffusion of vapour within the gaseous phase (the transport of adsorbed water is not accounted for here). The description of these phenomena results in a complex set of coupled differential equations [22-25]. The major disadvantage of this approach is the great number of input data that are required and the experimental difficulty related to their assessment.

104 Water transport is then usually described in a simplified way using a single equation as for liquid 105 permeation only (the two other motions are included). This approach was found to be valid (water 106 transport in the gaseous phase was found to be negligible compared liquid permeation) for weakly 107 permeable materials [24]: this was the case for a concrete with an intrinsic permeability equal to 1×10<sup>-</sup> 108 <sup>21</sup> m<sup>2</sup> [26]. More recently, Thiéry et al. [27, 28] estimated the respective contribution of each motion for three different concretes. They showed that this approach validity depends on the intrinsic permeability 109 110 and the RH-domain that is considered. For instance for a high-performance concrete with low permeability (~2×10<sup>-22</sup> m<sup>2</sup>) the assumption appears to be valid between RH=20% and 100% whereas 111 for a low-strength concrete with high permeability ( $\approx 4 \times 10^{-19} \text{ m}^2$ ) the validity domain is limited to 65%-112 113 100% RH.

To describe water transport in a simplified way, we have started using Darcy's law extended to
unsaturated flow [29] which allowed estimating the water flux *j*:

$$\underline{j} = -\rho \frac{K_e}{\eta} \underline{grad}(P) \tag{1}$$

116 where *P* is the liquid pressure [Pa];  $\eta$  and  $\rho$  the water viscosity [Pa·s] and density [kg/m<sup>3</sup>] respectively. 117  $K_e$  is the effective permeability that characterizes the resistance of the unsaturated concrete to water 118 flow (under a pressure gradient) [m<sup>2</sup>]. It was expressed as the product ( $K \times k_r$ ) in which *K* is the 119 intrinsic permeability [m<sup>2</sup>] and  $k_r$  the relative permeability that accounts for the effect of desaturation 120 on the depercolation of the saturated porous network (ranges between 0 and 1). The mass 121 conservation equation then wrote:

$$\frac{\partial}{\partial t}(\rho \phi S) = -div\left(\underline{j}\right) = div\left[\rho \frac{K_e}{\eta} \underline{grad}(P)\right]$$
(2)

S is the saturation index [without unit], it describes how pores are filled with liquid water and ranges between 0 (dry state) and 1 (saturated state).  $\emptyset$  is the concrete porosity (volume per volume) [without unit]. In isothermal conditions, assuming that water is incompressible and that a differentiable relation between saturation *S* and water pressure *P* exists (this relation is known as the capillary pressure curve) eq. (3) could be easily obtained:

$$\phi\left(\frac{\partial S}{\partial P}\right)\frac{\partial P}{\partial t} = div\left[\frac{K_e}{\eta}\underline{grad}(P)\right] - S\left(\frac{\partial\phi}{\partial t}\right) \tag{3}$$

127 The right-hand term  $\left(S\frac{\partial\phi}{\partial t}\right)$  accounted for porosity reduction induced by carbonation and the pressure 128 *P* was calculated using the so-called Kelvin-Laplace equation:

$$P(h) = -\rho \frac{RT}{M} ln(h) \tag{4}$$

129*R* is the universal gas constant [J/mol/K], *T* is the absolute temperature [K]. *M* is the liquid water molar130mass [kg/mol] and *h* is the relative humidity. The description of water transport within concrete thus131required the knowledge of only three physical parameters: the concrete porosity ( $\emptyset$ ), the effective132permeability ( $K_e$ ) and the left-hand term  $\left(\frac{\partial S}{\partial P}\right)$  which was assessed using the water retention curve [30].133It must be noted that for constant porosity, eq. (3) could be simplified to eq. (5) which was used to134describe unsaturated water transport in the non-carbonates pastes (part 4.4.1).

$$\phi\left(\frac{\partial S}{\partial P}\right)\frac{\partial P}{\partial t} = div\left[\frac{K_e}{\eta}\underline{grad}(P)\right]$$
(5)

# 135 **3. Materials and specimens**

### 136 **3.1. Materials**

137 Four different hardened cement pastes with constant water to binder ratio (0.40) were used (Table 1). 138 These binders were chosen because they are being used for R&D studies by the French Agency for 139 radioactive waste management (Andra) and its partners. The low-pH mix was designed in the field of 140 geological disposal to limit the chemical interaction between clay minerals and concrete structures [31, 141 32]. The CEM I, CEM V/A and Low-pH mix were already studied in a previous work [33, 34] in which 142 the water transport properties of the non-carbonated materials were characterized. The cement pastes CEM I, CEM III/A, CEM V/A and Low-pH blend (T1 from [32]) are respectively noted PI, PIII, PV and 143 144 PBP in this study.

#### Table 1: Cement pastes composition.

Compound	CEM I	CEM III/A	CEM V/A	Low-pH blend (T1)
CEMI	100%	39%	56%	37.5%
Slag	-	61%	22%	-
Fly ash	-	-	22%	30%
Silica fume	-	-	-	32.5%
Superplasticizer Chryso <sup>®</sup> Fluid Optima 175	-	-	-	1% of binder (by mass)
Water to binder ratio	0.40	0.40	0.40	0.40

146

## 147 3.2. Specimens preparation

148 The pastes were prepared in twelve consecutive batches of 2 L (3 batches for each composition) over 149 two days. The appropriate amounts of cement and water were mixed in a planetary mixer until 150 homogenization of the fresh mix. The paste was then poured into polypropylene cylindrical moulds 151 (Ø50×100 mm) and vibrated to remove entrapped air bubbles. The specimens were kept two weeks 152 after casting in their sealed moulds before unmoulding. The specimens were then cured for four 153 months in sealed containers immersed in specific curing solutions. The composition of the curing 154 solutions of the PI, PIII and PV specimens was designed to prevent calcium and alkalis leaching. The 155 pore solutions were expressed at high-pressure [35] from specimens kept in sealed moulds for 4 months. The use of the device optimized by Cyr & Daidié [36] allowed retrieving several millilitres of 156 157 solution, the composition of which was analyzed using ionic chromatography (Table 2). 158 Because the pore-solution of PBP was known to exhibit significant changes during the early months of hydration [32], a different protocol was chosen: several specimens were reduced into a rough powder 159 160 and added to deionized water to generate the curing solution.

- 161
- 162

#### Table 2: Ion chromatography results for the pore solutions of PI, PIII and PV.

Cations (mmol/L)	PI	PIII	PV
Na <sup>+</sup>	52	130	98
K <sup>+</sup>	473	366	469
Ca <sup>2+</sup>	2.5	2.3	2.0
рН	13.6	13.5	13.5

164 The top and bottom parts of the samples that present different properties from the bulk [37-39] were 165 cut off and discarded. The thickness of the parts to be removed was assessed by using accelerated

- 166 chemical degradation [40]. One specimen of each paste was immersed in ammonium nitrate solution
  167 (6 mol/L). The leaching solution was stirred constantly but not renewed [41]. After one month, the
  168 samples were withdrawn from the solution, sawn in two parts and sprayed with phenolphthalein
- 169 solution. Three zones could be observed (Figure 3):
- the central part where the degradation depth was constant versus height,
- the bottom part where the degradation depth was less due to sedimentation,
- the top part where the degradation depth was higher due to bleeding and sedimentation.
- 173 The removal of top and bottom ends (10 mm) was believed to yield homogeneous properties versus

height. 140 disks per paste (6 mm-thick) were sawn from the central part of the resulting specimens.

- 175 Most of these disks (110 per paste) were used for carbonation study and the remaining part was
- 176 dedicated to the non-carbonated pastes characterization.



Figure 3: Degradation profile of PI sample after one month in ammonium nitrate (6M) at
 ambient temperature.

# 179 3.3. Accelerated carbonation

180	Accelerated carbonation was achieved using the device developed by Drouet [33]. It includes a
181	climatic chamber for environmental control and a specific system allowing an accurate regulation of
182	the $CO_2$ partial pressure. Prior to carbonation, the 440 disks were kept in the carbonation device
183	without CO <sub>2</sub> (25°± 0.2°C and 55 ± 1% RH) in order to reach a constant value of saturation that
184	promotes carbonation [3, 42-44]. One month later, the equilibrium was reached (constant mass
185	variation) and the carbonation test was started with a $CO_2$ content of 3.0 ± 0.2%. This value was

- 186 chosen to ensure representativeness of the mineralogical evolution compared to atmospheric
- 187 carbonation [45]. The carbonation progress was monitored in a simple way through mass variation.
- 188 The process was pursued until the complete carbonation state which was verified afterwards using X-
- 189 ray diffraction (XRD) and thermogravimetric analysis (TGA) (see section 5.1.).

# 190 **4. Methods**

### 191 **4.1. Mineralogical assemblage**

- 192 The mineralogical changes induced by carbonation were identified using a PANalytical X'Pert
- 193 diffractometer and Cu Ka radiation ( $\lambda = 1,54$  Å) (XRD). The solid samples surface was scanned
- between 5 and 65°, with a step size of 0.017°. The use of the X'Celerator detector allowed the
- acquisition time of the diffractograms to be around 20 minutes.
- 196 Thermogravimetric analysis experiments (TGA) were accomplished using a NETZSCH STA 409 PC
- 197 LUXX device to determine the amount of portlandite (CH) and calcium carbonate ( $C\overline{C}$ ). Following
- 198 current methodology [46] the samples were powdered (120 ± 0.1 mg) and tested at a constant heating

199 rate of 10°C/min up to 1150°C under N<sub>2</sub> flowrate (80 mL/min).

- The C-S-H content was evaluated following the method proposed by Olson and Jennings [47] and the water content at equilibrium with RH = 20% (input data for the estimation process) was obtained from the water desorption isotherms.
- 203 <sup>29</sup>Si Magic Angle Spinning (MAS) Nuclear Magnetic Resonance (NMR) was used to probe the effect of carbonation on the C-S-H structure as commonly done [45, 48-51]. We did not use <sup>13</sup>C NMR because 204 it was found difficult to distinguish one calcium carbonate from the others [52]. The <sup>29</sup>Si MAS NMR 205 206 experiments were conducted following the current protocol of Brunet et al. [53]. Spectra were collected 207 using a Bruker 300WB NMR spectrometer operating at a Larmor frequency of 59.3 MHz (magnetic 208 field 7.05 T). Samples intended for MAS NMR analysis were powdered (around 100 mg sieved to 209 remove the particles larger than 100 µm) and packed in ZrO<sub>2</sub> 4mm (outer diameter) rotors. Sample 210 spinning frequency was 10,000 Hz and recycle delay was 2s. No differences in line shape were seen 211 for longer recycle delays (20s, 200s) and spin rate of 10 kHz was found to be sufficient to remove

paramagnetic effects (*i.e.* here spinning sidebands intensity minimized). Data were processed using
an in-house made software [53].

### 214 **4.2. Porosity**

215 Water porosity Ø was obtained using oven-drying. The specimens were preliminarily saturated under 216 vacuum and water following the current standard used in France [54]. 80°C was chosen as the 217 reference temperature in order to enable comparison with a previous study focused on similar 218 materials [33] but supplementary tests were conducted at 105°C to enable comparison with literature. 219 The pore size distribution was investigated by mercury intrusion porosimetry (MIP) using a 220 Micrometrics Autopore IV. Samples were crushed into small parts (several millimetres), frozen by 221 immersion into liquid nitrogen, let to dry under vacuum for seven days and then tested at 20 ± 2°C. 222 Two samples of each formulation were used.

# 223 **4.3. Water desorption isotherm**

224 The desorption isotherms were characterized using the desiccator method [55]. The non-carbonated 225 samples were tested just after the cure (after 4 months) whereas the carbonated ones were tested 226 after the cure (4 months) and carbonation (1 year). All the samples were resaturated before the test (4 h under vacuum and then 20 h under water and vacuum). A set of specimens (from 3 to 6) of each 227 228 paste was inserted into a desiccator above a specific saturated salt solution to control the RH (Table 229 3). Thirteen different sample-sets were placed simultaneously into thirteen different desiccators 230 including different salt solutions. This procedure allowed reducing the test duration, but might have 231 resulted in increasing variability.

232

#### Table 3 Saturated salt solutions used in the desiccator method [56-59].

Saturated salt solution	Chemical formula	RH (20°C)
Calcium chloride	CaCl <sub>2</sub>	≈3%
Lithium chloride	LiCI	11%
Potassium acetate	C <sub>2</sub> H <sub>3</sub> KO <sub>2</sub>	23%
Magnesium chloride	MgCl <sub>2</sub>	33%
Potassium carbonate	K <sub>2</sub> CO <sub>3</sub>	43%
Magnesium nitrate	Mg(NO <sub>3</sub> ) <sub>2</sub>	54%
Sodium bromide	NaBr	59%
Ammonium nitrate	NH <sub>4</sub> NO <sub>3</sub>	63%
Potassium Iodide	KI	70%

Ammonium chloride	NH <sub>4</sub> Cl	80%
Potassium nitrate	KNO <sub>3</sub>	92%
Potassium sulfate	K <sub>2</sub> SO <sub>4</sub>	98%
Deionized water	H <sub>2</sub> O	100%

The thirteen desiccators were put in an air-conditioned room ( $20 \pm 2^{\circ}$ C). At given times, the desiccators were opened and the samples weight was measured. The results (relative mass variation at equilibrium  $\left(\frac{\Delta m}{m}\right)(h)$  enabled the calculation of the water retention curves expressed in terms of water content w(h)(ratio of the mass of water for the RH *h* to the dry mass) [60]:

$$w(h) = \frac{d_{sat}}{d_{sat} - \emptyset} \left[ \left( \frac{\Delta m}{m} \right)(h) + \frac{\emptyset}{d_{sat}} \right]$$
(6)

where  $d_{sat}$  is the specific gravity of the saturated paste. It was measured using the buoyancy method.

239 The deviation from equilibrium was characterised using an indicator  $\tau$  defined as:

$$\tau(t) = \frac{\left(\frac{\Delta m}{m}\right)_{t+1} - \left(\frac{\Delta m}{m}\right)_t}{\left(\frac{\Delta m}{m}\right)_t}$$
(7)

where  $\left(\frac{\Delta m}{m}\right)_t$  and  $\left(\frac{\Delta m}{m}\right)_{t+1}$  are the relative mass variation measured at the times t and one day after respectively. Once the indicator  $\tau$  became lower than 0.05%, the equilibrium was considered achieved. For instance, in the case of carbonated PI at 43% RH (Figure 4), the criterion was reached after 110 days. After 275 days, the indicator  $\tau$  reached 0.01% (five time lower than the criterion) but the difference between the relative mass variations at 110 and 275 days remained very limited: about 3.5% (relative value).



Figure 4: Characterization of the desorption isotherm (carbonated PI at 20°C and 43% RH).

248 Complementary tests were conducted using a sorption balance (SMS DVS Advantage) [10, 61]. This 249 device presents two major benefits: (i) a complete desorption isotherm could be obtained in about ten 250 days and (ii) it allowed testing some specific RH values that are difficult to achieve using saturated salt 251 solutions (especially RH lower than 30%). A few disks were crushed and then powdered (sieved to 252 remove the particles larger than 100 µm) in a CO<sub>2</sub>-free glove box. The powder was resaturated using 253 deionized water: a sample of about 50 mg was taken and introduced into the sorption balance. The 254 tests were performed at 25°C ± 0.1°C and the RH was decreased by steps under the "dm/dt" mode 255 (the software automatically shifted from one RH step to another when equilibrium was considered to 256 be reached).

257 The specific surface area ( $S_s$ ) was assessed using the well-known BET model [62, 63]:

$$w(h) = \frac{Cw_m h}{(1-h)[1+(C-1)h]}$$
(8)

where *C* and  $w_m$  are the two BET parameters. *C* is related to the energy of the first layer and  $w_m$ represents the water content needed to complete a monomolecular layer. The specific surface area  $(S_S)$  was calculated using the BET monolayer parameters  $w_m$  following [64]:

$$S_S = N_A \frac{w_m A}{\rho V} \tag{9}$$

where  $N_A$  is Avogadro's number, V is the molar volume of water vapour and A is the surface occupied by one molecule of liquid water obtained following [65, 66]:

$$A = 1.091 \frac{M}{N_A \rho} \tag{10}$$

## 263 4.4. Permeability

#### 264 4.4.1. Inverse analysis

265 The intrinsic permeability (*K*) was fitted through numerical simulations to match experimental data [26,

266 67]. Here, initially saturated specimens (Ø50×100 mm) were submitted to 55% RH and 25°C in a

267 climatic chamber during 100 days. The finite-element code Cast3m<sup>1</sup> was used to solve eq. (3). The

- relative permeability to liquid water  $k_r$  was evaluated using the Mualem-van Genuchten model [68,
- 269 69]. The water retention curves were fitted using the equation proposed by van Genuchten:

<sup>&</sup>lt;sup>1</sup> <u>http://www-cast3m.cea.fr/changelang.php?lang=en</u>

$$w = w_{sat} \left[ 1 + \left(\frac{P}{P_0}\right)^{\frac{1}{1-m}} \right]^{-m}$$
(11)

- where  $P_0$  and m are the two positive parameters of the model and  $w_{sat}$  is the water content at
- saturation (RH = 100%). The relative permeability  $k_r$  was calculated using Mualem's model (the value
- of the parameters  $P_0$  and m are the same as above) [69]:

$$k_r = \left[1 + \left(\frac{P}{P_0}\right)^{\frac{1}{1-m}}\right]^{-mp} \left\{1 - \left(\frac{P}{P_0}\right)^{\frac{1}{1-m}} \left[1 + \left(\frac{P}{P_0}\right)^{\frac{1}{1-m}}\right]^{-m}\right\}^2$$
(12)

where the exponent *p* is known as the pore-interaction factor (without unit) and embodies pore
tortuosity. It was taken equal to +0.5 as suggested by Mualem [68], but *p* may be negative or positive
[68].

276 The moisture capacity (the left-hand term  $\phi(\frac{\partial S}{\partial P})$  in eqs. 3 and 5) was then:

$$\emptyset\left(\frac{\partial S}{\partial P}\right) = \frac{m\emptyset}{(m-1)P_0} \left(\frac{P}{P_0}\right)^{\frac{1}{1-m}} \left[1 + \left(\frac{P}{P_0}\right)^{\frac{1}{1-m}}\right]^{-1-m} \tag{13}$$

The numerical restitution (solid line) of the experimental relative mass loss (circles) is illustrated on Figure 5 (a). The value of *K* is estimated by minimization of the quadratic difference between the computed and measured relative mass loss for each measuring time (Figure 5 b). The minimum yielded the value of the intrinsic permeability. In this example (non-carbonated PI), the optimal value of *K* was equal to  $2.7 \times 10^{-22}$  m<sup>2</sup>.



282	Figure 5: (a) Numerical restitution of the relative mass loss as a function of the intrinsic
283	permeability $K$ (non-carbonated PI) and (b) Evolution of the quadratic deviation as a function of
284	the intrinsic permeability <i>K</i> .

Inverse analysis was used for non-carbonated specimens only. The carbonated ones were too small
(the specimens dry too quickly and the mass loss is too low to yield appropriate results) and another
approach had to be used to assess permeability.

#### 288 4.4.2. Cup-method

- 289 The cup-method [70] is a direct way of determination of the effective permeability. In this experiment,
- 290 disks do constitute a boundary separating two different environments (same temperature but two
- 291 different RH:  $h_1$  and  $h_2$ ) through which water is transported (Figure 6)



292

#### 293

Figure 6 : Sketch and photos of the cup-method set up.

The permeability was assumed to be constant on the RH range  $[h_1; h_2]$ . In steady state, Darcy's law allows estimating the effective permeability  $K_e$  based on the mass loss measurement (Figure 7):

$$K_e = -\frac{\dot{m}\eta}{\rho\Sigma\frac{\Delta P}{t}}$$
 and  $\Delta P = -\rho\frac{RT}{M}ln\left(\frac{h_1}{h_2}\right)$  (14)

296 where  $\dot{m}$  is the mass loss time derivative [kg/s],  $\Sigma$  the disk cross sectional area [m<sup>2</sup>], *t* the thickness 297 [m],  $\Delta P$  the liquid pressure difference between the two environments (computed using eq.14) [Pa].



Figure 7 : Cup mass evolution (PI disk) vs. time for a 64%  $(h_1)$  – 55%  $(h_2)$  RH range at 25°C

An experimental set-up was implemented by adapting existing diffusion cells (Figure 6). In practice,  $h_1$ was controlled using a saturated salt solution whereas  $h_2$  was controlled using a climatic chamber (which also allows controlling temperature). In this test, initially saturated samples are used to focus on the drying path (and  $h_1$  was always greater than  $h_2$ ). The cup-method is implemented on carbonated disks (due to the small specimen thickness) as well as non-carbonated ones to validate the method.

# 305 **4.5. Cracking**

Because cracks were observed on the disks surface after carbonation, some of them were impregnated with a fluid resin incorporating a fluorescent dye<sup>2</sup>. The disks were observed in the light of a ultra-violet (UV) lamp. The resin-filled cracks could clearly be seen thanks to the dye fluorescence. Pictures were taken and then processed using a specific software<sup>3</sup> in order to provide a cracking index ( $I_c$ ) to compare qualitatively the pastes (grey-scale image processing). The cracking index was simply defined as the ratio of the number of pixels attributed to the cracks to the total number of pixels of the disk surface:

$$I_{c} = \frac{Number \ of \ pixels \ attributed \ to \ the \ cracks}{Total \ number \ of \ pixels \ of \ the \ surface}$$
(15)

<sup>&</sup>lt;sup>2</sup> Struers EpoDye and EpoFix

<sup>&</sup>lt;sup>3</sup> http://imagej.nih.gov/ij/

## 313 **5. Results**

#### 314 **5.1. Mineralogical changes**

315 The usual mineralogical changes could be highlighted using XRD (Figure 8): dissolution of the main 316 hydrates (portlandite, AFt and AFm) and precipitation of calcium carbonate (calcite and vaterite, the 317 presence of aragonite was not identified). These evolutions were supported by the TGA data (Figure 318 9). 100 days were necessary to achieve a stabilized state of carbonation (constant concentration of 319 calcium carbonate). As it was expected, the amount of calcium carbonate increased with the initial 320 portlandite content. The remaining presence of portlandite was also observed after carbonation that 321 was attributed to the calcium carbonate formation around the portlandite crystal inhibiting their dissolution [71-74]. 322

Figure 10 presents the results obtained using <sup>29</sup>Si NMR. Q<sub>0</sub> represents isolated SiO<sub>2</sub> tetrahedra, Q<sub>1</sub> 323 denotes chain end groups,  $Q_2$  middle groups,  $Q_3$  branching sites and  $Q_4$  cross-linking ones [48].  $Q_2^p$ 324 represents the bridging SiO<sub>2</sub> sites but other tetrahedra cannot be distinguished using <sup>29</sup>Si MAS NMR. 325 A more detailed analysis would be required to describe accurately the C-S-H structure [51]. <sup>29</sup>Si NMR 326 327 spectra showed that carbonation decreases the amount of anhydrous phases  $(Q_0)$  in agreement with the XRD results. But above all this, the characteristic peaks of the C-S-H (Q1, Q2<sup>p</sup> and Q2) drastically 328 329 decreased revealing significant decalcification of the C-S-H. The end result was close to a silicate material (Q3 gel and Q4 gel) which could possibly contain calcium (Ca-enriched silica gel with a low C/S 330 331 ratio).





332 Figure 8: X-ray diagrams acquired for non-carbonated (NC) and carbonated (C) pastes PI, PIII, 333 PV and PBP - AFt: ettringite, AFm: monosulfate, m: mullite, p: portlandite, q: quartz, Gy: 334 gypsum, c: calcite, v: vaterite, a: aragonite, d: dolomite, an: anhydrous phases C<sub>2</sub>S and C<sub>3</sub>S, h: hematite. 335



336 Figure 9: Evolution of the amounts of portlandite (CH) and calcium carbonate (CC) as a function 337 of carbonation time in PI, PIII, PV and PBP.



Many disks were cracked or even broken at the end of the carbonation campaign, regardless of the composition: more than 50% of the PI specimens and to a lesser extent PIII, PV and PBP (between 25% and 40%). This was believed to be due to the combination of drying and carbonation shrinkage. Only the unbroken and non-cracked disks (as observed with the naked eyes) were used for further characterization.

# 344 **5.2.** Porosity

The precipitation of calcium carbonate led to the reduction of water porosity (Table 4). The fall of porosity was of the same order of magnitude of those obtained by Ngala and Page [5]. The porosity variation was directly related to clinker substitution by pozzolanic additions: the higher the initial portlandite content, the higher the fall of porosity (Figure 9 and Table 4).

349

Table 4: Porosity to water of the cement pastes.

		PI	PIII	PV	PBP
Motor contont	Non-carbonated	22.1%	26.2%	25.2%	30.6%
Water content w <sub>sat</sub>	Carbonated	10.0%	14.8%	14.1%	20.0%
	Non-carbonated	36.3%	39.8%	36.9%	41.0%
Porosity	Carbonated	21.1%	29.3%	27.6%	35.5%
-	Variation	-15.2%	-10.5%	-9.3%	-5.5%

Not only the total porosity was reduced but also the pore size distribution was modified (Figure 11).

353 The critical pore diameter decreased for PI and PV whereas a slight, but significant, coarsening of the

354 pore structure was observed for PIII and PBP.





Figure 11: Impact of carbonation on the pore size distribution (characterized by MIP two

356

samples per formulation).

# 357 5.3. Water desorption isotherm

358 Figure 12 presents the water desorption isotherms obtained using the desiccator method (filled circles)

and the sorption balance (DVS, open diamonds). A satisfactory agreement between the two methods

was then obtained for all the binders in the non- and carbonated states. Carbonation led to a
significant drop of water content at equilibrium that was directly related to porosity clogging (Table 4).
The desorption isotherms morphology was also altered as a consequence of the pore size distribution
modification. This is patent when saturation is used to plot the water retention curves (Figure 13). As
already beheld by Houst and Whittmann [4], the modifications of the CEM I paste curve appeared
moderate and remained limited to low RH. Those of the blended pastes were more significant and
appeared to be all the more significant as the amount of addition was high.



Figure 12: Water retention curves of the hardened cement pastes (water content). The lines are
 guides for the eyes only.



evaluated; the results are presented on Figure 13 and the parameters are listed on Table 5.



Figure 13: Water retention curves of the hardened cement pastes (saturation). The lines
 correspond to van Genuchten model (eq. 11).

#### Table 5: The van Genuchten parameters ( $P_0$ in MPa and m without unit) obtained by

375

	F	ין	F	2111 2111	F	νV	PE	3P
	$P_0$	т	$P_0$	т	$P_0$	т	$P_0$	т
Non-carbo	51.4	0.46	86.5	0.56	96.9	0.53	108.7	0.58
Carbo	26.8	0.33	14.9	0.34	20.8	0.34	14.9	0.43

376

Carbonation led to a reduction of the specific surface area (Table 6). Using the approach proposed by
Olson and Jennings [47] together with the desorption isotherm (water content at 20% RH), the C-S-H
content was estimated (

380 Table 7). It was then implicitly assumed that this approach remains valid for carbonated cementitious

381 materials. It was found that  $\Delta S_s$  increased with  $\Delta C$ -S-H indicating C-S-H carbonation and partial

382 dissolution.

#### Table 6: Impact of carbonation on the specific surface area ( $S_s$ ).

<i>S<sub>s</sub></i> (m²/g)	PI	PIII	PV	PBP
Non-carbonated	190	300	298	382
Carbonated	93	89	98	88
$\Delta S_S$	-97	-211	-200	-294

384

383

385

#### Table 7: Impact of carbonation on the C-S-H content.

C-S-H concentration (mol/L of paste)	PI	PIII	PV	PBP
Non-carbonated	5.2	6.5	6.5	7.6
Carbonated	3.7	3.3	3.9	2.6
∆C-S-H	-1.4	-3.2	-2.5	-5.0

386

# 387 **5.4. Cracking**

Figure 14 presents the pictures of the non- and carbonated disks surface. The absence of cracks on 388 389 the non-carbonated disks could be noticed whereas the carbonated ones were more or less 390 significantly cracked depending on the considered binder. Although only the disks surface could be 391 observed, the resulting cracking pattern was believed to be representative of that of the bulk. The 392 image processing results are reported on Figure 14 ("thresholded" binary image) and in Table 8 393 (cracking index values). An average crack opening of 10-15 µm was measured for all the binders. 394 Different values of the cracking index were found depending on the considered binder: PI presented 395 the lowest value and PBP the highest.



396

Figure 14: Photos of the non- and carbonated impregnated disks under UV light (on the and result of the image processing (on the right)

398

Table 8: Cracking indices (*I<sub>c</sub>*).

 PI
 PIII
 PV
 PBP

 I<sub>c</sub> (%)
 4%
 9%
 7%
 10%

399

# 400 5.5. Permeability

## 401 5.5.1. Inverse analysis

402 The numerical restitution of the experimental drying kinetics of the Ø50×100 mm cylinders (55% RH

403 and 25°C) is presented on Figure 15 (following the method presented in part 4.4.1). The obtained

404 intrinsic permeability values are compiled in Table 9. These values are consistent with the pore size

405 distributions: the finer the pore size distribution, the lower the permeability value.



407 Figure 15: Numerical restitutions of the relative mass loss as a function of the intrinsic
 408 permeability (*K*) (PI, PIII, PV and PBP).

409

Table 9: Values of the intrinsic permeability (*K*) of non-carbonated paste.

	PI	PIII	PV	PBP
$K_{nc}$ (×10 <sup>-22</sup> m <sup>2</sup> )	2.70	0.26	0.50	0.45

410

#### 411 **5.5.2. Cup method**

412 Firstly, the cup-method was applied on non-carbonated PI to compare with inverse analysis (Figure

413 16). A good agreement between both methods was noticed: the two datasets presented the same

414 order of magnitude and described a similar trend. A small difference could however be observed that

- 415 could be due to:
- 416 the variability linked to the differences between the batches used for the two methods (the data
- 417 required for inverse analysis were extracted from a previous study [33],
- 418 uncertainties on the unsaturated properties (for instance water retention curve),
- 419 the error on the relative permeability  $(k_r)$  assessed using Mualem's model and more specifically
- 420 the value of the pore interaction factor p (eq. 12) [68, 75, 76].



# 421 Figure 16: Effective permeability ( $K_e$ ) evolution vs. saturation state (S), at 25°C, for non-422 carbonated Pl.

423 It was noticed that for RH lower than 40%, a permeability increase was obtained for the non-424 carbonated specimens. Cracks could be observed on the disk surface. They were believed to be due 425 to restrained drying shrinkage. Beyond that, the authors think that the comparison was globally 426 satisfactory and that the cup method is an efficient tool for unsaturated permeability evaluation. This 427 statement could not however be generalized to the blended cements (Figure 17) for which the reliable 428 permeability description was limited to a small range of saturation (around 0.8 to 1.0). This was due to 429 their refined pore size distribution leading to flattened desorption isotherms at high RH (Figure 12). In 430 the following sections, the permeability of non-carbonated blended cement pastes was then described 431 using inverse analysis.





The cup-method was applied to assess the unsaturated permeability of the carbonated pastes (Figure 18). A decrease in the permeability of PI was observed subsequently to carbonation that is consistent with porosity clogging. On the opposite side, a significant increase of permeability was found for the blended cements despite the fall of porosity.

#### — Carbonated disks — Non-carbonated disks



Figure 18: Impact of carbonation on the effective permeability ( $K_e$ ). The lines stand for the best fit using Mualem-van Genuchten model (with  $p \neq 0.5$ ).

Intrinsic permeability values of the carbonated pastes were assessed following eq. (16). This is the well-known Mualem-van Genuchten equation. It is the same as eq. (12) but it is expressed in terms of saturation instead of pressure. No restriction was imposed to the pore interaction factor value (the exponent p) to improve the fitting capacity [75-79]. The resulting p values were different from the default value (+0.5) proposed by Mualem [68]: they were all negative as it was already observed [75, 76]. The corresponding values (pore interaction factor and intrinsic permeability) are compiled in 448 Table 10.

$$K_e = K_c^{exp} S^p \left[ 1 - \left( 1 - S^{1/m} \right)^m \right]^2$$
(16)

449

450

Table 10: Intrinsic permeability value of the carbonated pastes.

	PI	PIII	PV	PBP
$K_c^{exp}$ (×10 <sup>-22</sup> m <sup>2</sup> )	1.16	74.82	74.39	81.48
р	-3.31	-2.59	-1.83	-2.18

451

452 Two different explanations were explored to explain the significant permeability increase of the
453 blended cement pastes: (*i*) coarsening of the pore structure and formation of large pores and (*ii*)
454 microcracking. These points are discussed in the following section.

# 455 6. Discussion

The coarsening of the pore structure and formation of large pores could not be observed using MIP. Microcracking was then believed to be the major cause of the permeability increase. This was supported by the fact that the permeability of the carbonated disks increases with the cracking index  $(I_c)$  as depicted on Figure 19 (a).  $R_K$  is the ratio of the intrinsic permeability of the carbonated paste  $K_c^{exp}$  to the non-carbonated one  $K_{nc}$  (Table 11):

$$R_K = \frac{K_c^{exp}}{K_{nc}} \tag{17}$$

For  $R_K \leq 1$  (PI), the high amount of calcium carbonates (due to the high initial portlandite content) led 461 to significant porosity clogging that prevailed over the effect of microcracking. On the contrary, for 462 463  $R_{K} \geq 1$  (PIII, PV and PBP), the effect of microcracking prevailed over porosity clogging. This fact was 464 directly related to the initial mineralogical composition of the pastes. C-S-H rich pastes (with low 465 portlandite) showed significant cracking after carbonation (Figure 19, b): the higher the initial C-S-H content, the higher the cracking index  $(I_c)$ . The authors believe that the cracking was induced by 466 C-S-H carbonation following the proposition of Swenson and Sereda [71]. The C-S-H decalcification 467 induced by carbonation and the subsequent polymerisation (increase of the main silica chain length) 468 generated shrinkage [80]. This carbonation shrinkage [42, 71, 81-83] eventually led to cracking as it 469

470 was already observed [20, 84, 85]. The substantial C-S-H decalcification observed for all the binders



471 after complete carbonation using <sup>29</sup>Si NMR supported this assumption (Figure 10).

Figure 19: Influence of cracking on permeability (a) and influence of C-S-H initial content on the cracking index ( $I_c$ ).

474 The dataset obtained was used to describe the effect of porosity clogging on permeability. A simple
475 law derived from Kozeny-Carman model [86-89] was used:

$$K_c^{K-C} = K_{nc} \left(\frac{\phi_c}{\phi_{nc}}\right)^3 \left(\frac{1-\phi_{nc}}{1-\phi_c}\right)^2 \tag{18}$$

where  $K_{nc}$ ,  $K_c^{K-C}$ ,  $\phi_c$  and  $\phi_{nc}$  are the intrinsic permeability and water porosity values of the non- and 476 477 carbonated pastes respectively. The ratio  $R_K$  was computed once again, but this time the permeability of the carbonated pastes was estimated using eq. (18) ( $K_c^{K-C}$ , Table 11). The results are depicted on 478 479 Figure 20. For the PI paste the measured permeability was of the same order as the one obtained using eq. (18) but was three times higher. This suggested that the effect of porosity clogging was 480 481 counterbalanced by microcracking. The results were however very different for the blended cements, 482 the discrepancy between experimental and computed data increased with the cracking index  $(I_c)$ . This 483 suggested that in that specific case microcracking was the main cause of permeability increase and 484 that clogging had negligible impact.

Permeability (x10 <sup>-22</sup> m <sup>2</sup> )	PI	PIII	PV	PBP
K <sub>nc</sub>	2.70	0.26	0.50	0.45
$K_c^{exp}$	1.16	74.82	74.39	81.48
$K_c^{K-C}$ (eq. 18)	0.35	0.08	0.16	0.24



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489

Fi

Figure 20: Competition between porosity clogging and cracking.

# 490 Conclusion

491 The impact of carbonation on unsaturated water transport was addressed using four pastes made with 492 different binders (three commercial cements and a low-pH blend). In the framework of a simplified 493 approach for the description of unsaturated water transport, three physical parameters were 494 characterised: porosity, water desorption isotherm and permeability. The samples were carbonated at 495 a CO<sub>2</sub> content of 3% to ensure representativeness and mitigate cracking (as already observed at 496 50%). The precipitation of calcium carbonate led to porosity reduction, the extent of which was related 497 to the initial composition of the paste (portlandite and C-S-H contents). The water desorption curves 498 were significantly altered by carbonation:

499

• a drop of water content was observed over all the RH range according to porosity clogging;

• the isotherms morphology was changed in relation to pore size distribution modification.

501 The cup-method test was a good alternative to inverse analysis for the assessment of the unsaturated 502 permeability. The results highlighted a competition between two concomitant phenomena: porosity 503 clogging and microcracking. A decrease of permeability after carbonation was observed for Portland

- 504 cement (CEM I): porosity clogging prevailed over microcracking. On the contrary and despite the fall of
- 505 porosity, significant permeability increase was obtained for the blended cements: microcracking
- 506 prevailed over clogging. Permeability was found to increase with cracking (digital image processing)
- 507 and more specifically with the initial C-S-H content. C-S-H decalcification revealed by <sup>29</sup>Si NMR was
- 508 believed to be the main cause of shrinkage resulting in cracking.
- 509 This study was only a first step; these tests should also be conducted using concretes. The presence
- 510 of aggregates might help mitigating the consequences of carbonation shrinkage and change the
- 511 cracking pattern (the cracks could be concentrated around the aggregates).

# 512 Acknowledgments.

- 513 The authors would like to thank Andra for financial support. Lafarge, Calcia, Holcim kindly accepted to
- 514 supply cements free of charge.

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